

Department of Building & Grounds Architectural Services Division City of Baton Rouge Parish of East Baton Rouge

P.O. Box 1471 Baton Rouge, Louisiana 70821 225 389-4694 Voice 225 389-4704 Fax

ADDENDUM #2

November 4, 2024

TO ALL BIDDERS

PROJECT: BATON ROUGE POLICE DEPARTMENT TRAINING FACILITY CITY PARISH PROJECT NO. 21-ASC-CP-1560

The following revisions shall be incorporated in and take precedence over any conflicting part of the original contract documents.

- Clarification: A non-mandatory Pre-Bid Conference was held on Tuesday, October 22, 2024 at 2:00 PM at the Baton Rouge Police Department Training Facility future project site, 999 W. Irene Rd., Zachary, Louisiana, 70791. The meeting minutes and sign-in sheet are attached to this addendum.
- 2. Clarification: Attached are examples of Certified Payroll Reports for reference.
- 3. Project Manual, Bid Form. Delete Bid Form. Add attached Bid Form.
- 4. Project Manual. Add attached Unit Price Form.
- 5. Project Manual, Notice to Contractors, 2nd page, 1st paragraph: delete "**EBE goal of 7% of the contract amount**", substitute "**EBE goal of 22% of the contract amount**".
- Specification Section 01 0000.1.03.B.4, "City Parish Summary of Work; Schedule of Alternates": Delete "Schedule of Alternates: None", substitute "Alternate No. 1: Delete prefinished ceiling liner panels at the Simulator Building for the lump sum <u>DEDUCT</u> of:" and "Alternate No. 2 Delete the Classroom Building in its entirety for the lump sum <u>DEDUCT</u> of:"
- 7. See attached Addendum from BBI Architects, AAC (27 pages.)

The following revisions shall be incorporated in and take precedence over any conflicting part of the original contract documents.

FAILURE TO INDICATE RECEIPT OF THIS ADDENDUM ON BID FORM MAY BE CAUSE FOR THE BID TO BE REJECTED

Rob Gray, AIA, LEED AP BD+C, Interim Chief Architect Architectural Services Division 1100 Laurel Street, Rm. 227 Baton Rouge, LA 70802

LOUISIANA UNIFORM PUBLIC WORK BID FORM

TO: City of Baton Rouge Parish of East Baton Rouge Purchasing Division Room 826 City Hall 222 St Louis St Baton Rouge, LA 70802 BID FOR: Baton Rouge Police Department Training Facility 999 West Irene Rd. Zachary, LA 70791

City-Parish Project No. 21-ASC-CP-1560

The undersigned bidder hereby declares and represents that she/he; a) has carefully examined and understands the Bidding Documents, b) has not received, relied on, or based his bid on any verbal instructions contrary to the Bidding Documents or any addenda, c) has personally inspected and is familiar with the project site, and hereby proposes to provide all labor, materials, tools, appliances and facilities as required to perform, in a workmanlike manner, all work and services for the construction and completion of the referenced project, all in strict accordance with the Bidding Documents prepared by: **Department of Buildings & Grounds, Architectural Services Division** and dated: **October 1, 2024**.

Bidders must acknowledge all addenda. The Bidder acknowledges receipt of the following **ADDENDA:** (Enter the number the Designer has assigned to each of the addenda that the Bidder is acknowledging)

TOTAL BASE BID: For all work required by the Bidding Documents (including any and all unit prices designated "Base Bid" * but not alternates) the sum of:

Dollars	(\$)
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ALTERNATES: For any and all work required by the Bidding Documents for Alternates including any and all unit prices designated as alternates in the unit price description.

Alternate No. 1: Alternate No. 1: Delete prefinished ceiling liner panels at the Simulator Building for the lump sum DEDUCT of

	Dollars	(\$)
Alternate No. 2: Alternate No. 2 Delete the Classroom Building in its	s entirety for the lum	p sum <u>DEDUCT</u> of
	Dollars	(\$)
Alternate No. 3: N/A		
	Dollars	(\$)
NAME OF BIDDER:		
ADDRESS OF BIDDER:		
NAME OF AUTHORIZED SIGNATORY OF BIDDER:		
TITLE OF AUTHORIZED SIGNATORY OF BIDDER:		
SIGNATURE OF AUTHORIZED SIGNATORY OF BIDDER **:		
DATE:		

THE FOLLOWING ITEMS ARE TO BE INCLUDED WITH THE SUBMISSION OF THIS LOUISIANA UNIFORM PUBLIC WORK BID FORM:

* The <u>Unit Price Form</u> shall be used if the contract includes unit prices. Otherwise it is not required and need not be included with the form. The number of unit prices that may be included is not limited and additional sheets may be included if needed.

** A CORPORATE RESOLUTION OR WRITTEN EVIDENCE of the authority of the person signing the bid for the public work as prescribed by LA R.S. 38:2212(B)(5).

BID SECURITY in the form of a bid bond, certified check or cashier's check as prescribed by LA RS 38:2218(A) is attached to and made a part of this bid.

LOUISIANA UNIFORM PUBLIC WORK BID FORM

UNIT PRICE FORM

TO: City of Baton Rouge Parish of East Baton Rouge Purchasing Division, City Hall 222 Saint Louis St., 8th floor, Room 826 Baton Rouge, Louisiana 70802 BID FOR: Baton Rouge Police Department Training Facility 999 West Irene Road Zachary, LA 70791

UNIT PRICES: This form shall be used for any and all work required by the Bidding Documents and described as unit prices. Amounts shall be stated in figures and only in figures.

DESCRIPTION:	🛛 Base Bid or 🕻	Alt.# Limestone and	l site preparation asso	ociated with installation of limestone.
REF. NO.	QUANTITY:	UNIT PRICE EXTENSION (Quantity times Unit Price)		
Unit Price #1	1000	Square Feet		

DESCRIPTION:	□ Base Bid or □ Alt.# Not applicable.							
REF. NO.	QUANTITY:	UNIT OF MEASURE:	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times Unit Price)				
Not applicable	Not applicable	Not applicable	Not applicable	Not applicable				

DESCRIPTION:	□ Base Bid or □ Alt.# Not applicable.							
REF. NO.	QUANTITY:	UNIT OF MEASURE:	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times Unit Price)				
Not applicable	Not applicable	Not applicable	Not applicable	Not applicable				

DESCRIPTION:	□ Base Bid or □ Alt.# Not applicable.							
REF. NO.	QUANTITY:	UNIT OF MEASURE:	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times Unit Price)				
Not applicable	Not applicable	Not applicable	Not applicable	Not applicable				

DESCRIPTION:	□ Base Bid or □ Alt.# Not applicable.							
REF. NO.	QUANTITY:	UNIT OF MEASURE:	UNIT PRICE	UNIT PRICE EXTENSION (Quantity times Unit Price)				
Not applicable	Not applicable	Not applicable	Not applicable	Not applicable				

DESCRIPTION:	□ Base Bid or □ Alt.# Not applicable.							
REF. NO.	QUANTITY: UNIT OF MEASURE: UNIT PRICE UNIT PRICE EXTENSION (Quantity times Unit Price)							
Not applicable	Not applicable	Not applicable	Not applicable	Not applicable				

All quantities are estimated. The contractor will be paid based upon actual quantities as verified by the Owner.

Certified Payroll Transcript

PR #1

Period: 1/30/2023 - 2/5/2023

Job: PSC H2 BUILDING RENOVATION FOR VETERAN'S AFFAIRS

Contract:

21-ASC-CP-1558

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						Но	ours					Ducient		Veek Ending 2/5	/23)
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Operator									Other No	n Taxable	_	0.00	Additional Medicare	Ta	
EEO:									Project T	otal		178.80	Louisiana Withholdin	g 19.07	
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Job Tot	als											Project	Total		')
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Cash Fring	e	0.00	0.00	0.00	0.00	0.00	(0.00	0.00	C	0.00	25.52	Social Security	145.33	
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5/17/2023 Date

١,

(Name of Signatory Party)

do hereby state:

(1) That I pay or supervise the payment of the persons employed by

(Title)

	(Contractor or Subcontractor) on the	
PSC H2 BLDG RENO	OVATIONS FOR VETERA	N'S AFFAIRS
	(Building or Work)	
that during the payroll perio	od commencing on the	30
day of January ,	2023_ , and ending the	5 day of
February , 20)23	
	aid project have been paid ve been or will be made eith	
	(Contractor or Subcontractor)	
person, other than permise (29 CFR Subtitle A), issue	ectly or indirectly from the full sible deductions as defined ed by the Secretary of Labo 948, 63 Stat. 108, 72 Stat. bed below:	in Regulations, Part 3 or under the Copeland
	s otherwise under this co priod are correct and complet	

for laborers or mechanics contained therein are not less than the applicable wage rates contained in any wage determination incorporated into the contract; that the classifications set forth therein for each laborer or mechanic conform with the work he performed.

(3) That any apprentices employed in the above period are duly registered in a bona fide apprenticeship program registered with a State apprenticeship agency recognized by the Bureau of Apprenticeship and Training, United States Department of Labor, of if no such recognized agency exists in a State, are registered with the Bureau of Apprenticeship and Training, United States Department of Labor.

(4) That:

(a) WHERE FRINGE BENEFITS ARE PAID TO APPROVED PLANS, FUNDS, OR PROGRAMS

Π-In addition to the basic hourly wage rates paid to each laborer or mechanic listed in the above referenced payroll, payments of fringe benefits as listed in the contract have been or will be made to appropriate programs for the benefit of such employees, except as noted in Section 4(c) below.

(b) WHERE FRINGE BENEFITS ARE PAID IN CASH

 \overline{M} - Each laborer or mechanic listed in the above referenced payroll has been paid, as indicated on the payroll, an amount not less than the sum of the applicable basic hourly wage rate plus the amount of the required fringe benefits as listed in the contract, except as noted in Section 4(c) below.

(c) EXCEPTIONS

EXCEPTION (CRAFT)	EXPLANATION					
REMARKS:						
NAME AND TITLE						
	SIGNATURE					
THE WILLFUL FALSIFICATION OF ANY OF THE ABOVE GTATEMENTS MAY SUBJECT THE CONTRACTOR OR SUBCONTRACTOR TO CIVIL OR CRIMINAL PROSECUTION. SEE SECTION 1001 OF TITLE 18 AND SECTION 231 OF TITLE 31 OF THE UNITED STATES CODE.						



BRPD Training Facility

999 West Irene Road Zachary, LA

Pre-Bid Meeting Minutes

- 1. Sign sheet was distributed
- 2. Rob Gray opened the meeting at 2pm on Tuesday, October 22, 2024 at the future site of the new buildings and presented the following:
 - a. Project: Baton Rouge Police Department Training Facility
 - Bid Opening is 2pm, Thursday, November 7th at Purchasing office at City Hall downtown on St. Louis Street
 - c. Bids can be submitted in person prior to the bid opening or on-line through Central Auction House
 - d. Bid Documents are available thru Central Bidding (centralauctionhouse.com)
 - e. Plans can be distributed individually however you will not be on the Central Auction House notification list.
 - f. Contract Completion Time: 180 consecutive days
 - g. Liquidated Damages: \$605 per day
 - h. Bid Bond: 5% of the bid
 - i. Project consist of:
 - i. 6,000sf Simulator building
 - ii. 2,400sf Classroom building
 - j. Bid Submission:
 - i. Hand-deliver to purchasing or submit via Central Auction House
 - ii. Bid documents available for download via Central Bidding
 - iii. Carefully follow the instructions on bidding included in the Project Manual.
 - k. Addenda(s):
 - i. Contractors are responsible for checking the bidding houses website for the addendums. They will not be sent to everyone directly.
 - ii. Addendum No 1 will be loaded tomorrow
 - 1. This Addenda will replace the entire set of drawings.
 - iii. There will be a second Addendum loaded 72 hours prior to bidding, 2pm November 4th



- I. Bidders requiring clarification or interpretation of the Construction Documents are to make a written request to the Architect by 2pm October 31st.
- m. Prior Approvals are to made via written request to the Architect by 2pm October 31st
 - i. Contractors were cautioned to review the specifications for specifics on how and what to submit for prior approval.
- n. No substitutions will be allowed after bidding
- o. In order to bid Contractors must be licensed in State of Louisiana for <u>Building</u> <u>Construction</u>
- p. Statutory Forms are to be submitted with the bid.
- 3. Brent Bueche reviewed the Scope of Work:
 - a. Simulator Building Purpose: Training facility for various police and SWAT scenarios.
 - i. Features:
 - 1. Multiple rooms and doors for different configurations
 - 2. Catwalk for observation
 - 3. Simulation rounds (not live fire)
 - 4. Open air ventilation with large fans (no air conditioning)
 - 5. Steel catwalk requiring detailed inspection by steel contractors
 - b. Classroom Building
 - i. Features:
 - 1. One large classroom
 - 2. Two restrooms
 - 3. Storage room
 - 4. Porch with large fans
 - c. Utilities:
 - i. Power from a nearby utility pole
 - ii. Water from the adjacent road
 - iii. Sewer treatment plant will be installation as part of the work
 - iv. Limestone driveway and parking lot
- 4. Unique Aspects of the project;
 - a. Complete sets of replacement windows and doors are to be included to be stored on-site.
 - b. Hallway Plugs: Blanks to create dead-end corridors for simulations.
 - c. Site Conditions: Poor soil quality will require significant excavation and backfilling.
- 5. Meeting was open to questions:
 - a. Project is not tax exempt
 - b. Prevailing wages under Davis-Bacon are not applicable.
- Certified Payroll will be required for federal audit compliance under the ARPA program. Submission
 of weekly payroll reports from general contractors and subs, even if showing zero dollars when work
 is paused
- 7. Excavation spoils may be stored on-site. Information will be provided in the next addendum

Meeting Adjourned

Project: BRPD Training Facility

Project No: <u>21-ASC-CP-1560</u>

Bid Date: November 7, 2024

PRE-BID CONFERENCE SIGN-IN SHEET

(Print) Name & Email	Phone Number	Fax Number	Company Name & License#	Signature
Name: Budgene BLANC Email: Bidge Mclinconstryst	225-435-30 000 mm	006	42839	Dyrette
Name: BRANDON LABIT Email: BRANDON CPREMICECONSTRUCT	225-384-0769 1 6c. co m		P.C.R.	S
Name: Frank Alvarce Email: FFANK @ Premier construction 6	r.coM 225.78.3665		44883 Premier Construction GRende	Inna alun
Name: Ricky Roth Email: Ricky Stricklin and Borter.com	601-207-6057		72413	Rufylah
Name: Michael Stricklin Email: MStricklin@Stricklinandpacte	s.Cam 318-4214860	1	72413 Stsjelmandgaster Can Street	h h

Project: BRPD Training Facility

Project No: <u>21-ASC-CP-1560</u>

Bid Date: November 7, 2024

PRE-BID CONFERENCE SIGN-IN SHEET

(Print) Name & Email Name: Cindy Crumhott	Phone Number	Fax Number	Company Name & License#	Signature
Email: Cindy C jread constructors,	net 225.201-8826		37085	Cincly Crumport
Name: GREG FLETTER Email: BEDS@JWGRAND.COM Name: Eric Turner	225 -767-3724		9569 JW GRAND	Jalj-
Email: ETURNERG UBSERTERPRISE, Com	205-936-9431		61043	En G
Name: Scor Deum. RE Email: Name: BEN FONTAINA	225 768-2943		43152	
Email: a pittman ecapcon la.com	225-751-0386		44097	- BA

Project: BRPD Training Facility

Project No: 21-ASC-CP-1560

Bid Date: November 7, 2024

PRE-BID CONFERENCE SIGN-IN SHEET

()	(Print) Nam & Email	Phone Number	Fax Number	Company Name & License#	Signature
_	Name: Linwood Williams	225)454 8903		SMR-civ. 1 contractors	Lonogo allo
	Email: Linwood 5m RCompanies	·		69088	
	Name: KENNY LANG	225.4 3.053		PRODIGY GROUP	A
_	Email: KMLCOX.NET				
	Name: Chad Bosson				\sim
	Email: Chadesandeac.com	225-315-1398		StEtleatings Air	Chill 2
_	Name: Chilg Varge				
•	Email: Carlos Vargos 5398 & gmain	225-413-37	86	Vargas Rainly LLC	Cal Van
-	Name: Manuel Manda				
-	Email: Constructionquality Lalognil.com	225-620-534	2	Construction Quality 240	Aproved Jondells

Project: BRPD Training Facility

Project No: <u>21-ASC-CP-1560</u>

Bid Date: November 7, 2024

PRE-BID CONFERENCE SIGN-IN SHEET

(Print) Name & Email	Phone Number	Fax Number	Company Name & License#	Signature
Name: Dave Davis	<u>.</u>			
Email: ddavis 91 @mc.con	225. 505.60	6	Apex Arditectures (Floor	s - Co-
Name: ROBERT THORNTON			LEPACY RESTONATION	
Name: Rogent Twornton Email: NWILLAMSE LEGNCYRANDR. CON Name: CHEEL MEDLOCK	504-421-729	3	and Referra	Ligand
Name: CHIET MESLOCK				
Email: Chetalezz Althor.co	m (22) 803-93	772	CMMETHLS	
Name:				
Email :				
Name:				
Email :				

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Built On Strong Foundations

Environmental • Construction Materials Testing • Geotechnical • Subsurface Investigations





Proposed Baton Rouge Police Department

SIM Shooting House

Zachary, Louisiana

Report of Subsurface Investigation

and Geotechnical Evaluation

Prepared For: Department of Building and Grounds – Architectural Services Division City of Baton Rouge

> Baton Rouge, Louisiana April 29, 2024



Tel: (225) 751-1727 Fax: (225) 752-1467 www.soearth.com

April 29, 2024

DEPARTMENT OF BUILDING AND GROUNDS – ARCHITECTURAL SERVICES DIVISION CITY OF BATON ROUGE 1100 Laurel Street – Room 227 Baton Rouge, Louisiana 70802

ATTENTION: Mr. Robert Gray, AIA, LEED AP BD+C

REFERENCE: Report of Subsurface Investigation and Geotechnical Engineering Evaluation Proposed BRPD SIM Shooting House Zachary, Louisiana SES Project No: B24-031

Dear Mr. Delaune:

Southern Earth Sciences, Inc (SES) has completed the subsurface investigation and geotechnical engineering evaluation for the referenced project. This report presents our understanding of the available project information and outlines our soil-related recommendations and comments regarding construction of the proposed new structures and surrounding pavement areas.

We appreciate this opportunity to be of service. Please do not hesitate to contact us if you have any questions.

Sincerely,

SOUTHERN EARTH SCIENCES, INC.

Kenny Meyn, P.E. Regional Manager Registered, Louisiana 24945

KΜ

Attachments



Report of Subsurface Investigation and Geotechnical Engineering Evaluation Proposed BRPD SIM Shooting House Zachary, Louisiana SES Project No: B24-031 April 29, 2024

1.0	Project Information1 -									
2.0	Site Description									
3.0	Field Investigation 1 -									
4.0	Laboratory Testing Program 2 -									
5.0	Subsurface Conditions 2 -									
5.1	Groundwater 2 -									
5.2	Presence of Expansive Soils3 -									
6.0	Foundation Recommendations 3 -									
6.1	Site Preparation 3 -									
6.2	Structural Fill Recommendations 4 -									
6.3	Shallow Foundations 5 -									
6.	3.1 Building Floor Slab 6 -									
6.	3.2 Grade Supported Slab Precautions 6 -									
7.0	Pavement Recommendations 7 -									
7.1	Pavement Base Course 8 -									
7.2	Alternative Base Course Options 8 -									
7.3	Moisture Control and Drainage for Pavements 8 -									
8.0	General Comments and Limitations9 -									

APPENDIX

Test Location Plan CPT Sounding Logs Soil Boring Logs



Report of Subsurface Investigation and Geotechnical Engineering Evaluation Proposed BRPD SIM Shooting House Zachary, Louisiana SES Project No: B24-031 April 29, 2024

1.0 PROJECT INFORMATION

Based on our understanding of the provided information, the project will consist of the design and construction of 2 new single-story structures with associated pavement areas. Based on the provided site plan, it is assumed that the new structures will encompass areas ranging from approximately 2,500 to 5,000 square feet. The proposed project site is located at 999 West Irene Road in Zachary, Louisiana.

No detailed loading, grading, and/or topographic information was available at the writing of this report; however, SES assumes that the maximum column and wall loads will not exceed 24 kips and 2.0 kips per linear foot, respectively. Furthermore, SES assumes less than two (2) feet of structural fill material will be required to achieve design grade for this project.

2.0 SITE DESCRIPTION

At the time of our fieldwork, the project site consisted of a relatively flat grass covered lot. Based on limited historical imagery from Google Earth which dates back to 1989, the site was heavily wooded up until late 2007 when it appears to have been cleared. Our scope of work did not include exploration to determine if abandoned foundations from previous development are located on the site.

3.0 FIELD INVESTIGATION

Three (3) Cone Penetrometer Test (CPT) soundings and three (3) auger boring were performed within the proposed structure and pavement areas at client determined locations. The test locations were identified in the field by SES personnel using the provided site plan and reference to onsite features. The locations were marked and recorded using a handheld GPS accurate to 25 feet. A Test Location Plan is attached in the **Appendix**.

The CPT soundings were advanced to depths of about 25 feet below existing site grades in general accordance with ASTM Specification D5778 using a track mounted Geoprobe 6625 Electronic CPT rig. CPT Log sheets graphically showing the cone tip resistance, friction, equivalent N-value and interpreted soil type at the sounding location is attached in the **Appendix**. Soil classifications were interpreted from methods recommended by Robertson and Campanella. Correlations between Cone Resistance values and Standard Penetration Testing "N" values were performed according to the methods developed by Robertson, Campanella and Wightman. The soil types and stratigraphy shown on the CPT Log sheets are based upon material parameters measured and evaluated as the cone is advanced.

The auger borings were performed to depths of about 10 feet below grade. The samples were obtained using a flight auger and were logged and bagged and identified according to project number, test location, and depth then transported to the laboratory for testing. The depths at which samples were obtained are



Report of Subsurface Investigation and Geotechnical Engineering Evaluation Proposed BRPD SIM Shooting House Zachary, Louisiana SES Project No: B24-031 April 29, 2024

shown on the attached soil boring logs of this report. All reference to depth has been made with respect to the existing ground surface encountered during our field investigation.

4.0 LABORATORY TESTING PROGRAM

A supplemental laboratory testing program was conducted to determine additional pertinent engineering characteristics of the subsurface materials. This program included visual description and classification and determination of the moisture content (ASTM D2216 *Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass*) on all samples. Furthermore, selected samples were subjected to Atterberg Limit Determinations (ASTM D4318 *Standard Test Methods for Liquid Limit, Plastic Limit and Plasticity Index of Soils*). The results of these tests are found in the accompanying soil boring logs located in the **Appendix**.

5.0 SUBSURFACE CONDITIONS

The following is a general description of subsurface conditions encountered during our site investigation. The descriptions provided below are brief and generalized. Additional details on soil layers and the collected data at each test location can be found on the individual CPT Sounding and Soil Boring Logs attached in the **Appendix**. All reference to depth has been made with respect to the existing ground surface encountered during our field investigation.

Subsurface soils encountered across the project site generally consist of medium stiff clays to a depth of about 20 feet, the maximum depth explored. Additional details on the collected data at each test location can be found on the individual CPT Sounding and Soil Boring Logs attached in the **Appendix**.

5.1 Groundwater

Groundwater levels were not encountered at the time of our field investigation. All test locations collapsed upon auger/rod removal at depths ranging from 2.1 to 3.2 feet below grade. Groundwater levels and the dates measured can be seen on the CPT Sounding and Soil Boring Logs in the **Appendix**.

Groundwater depths or elevations should be verified at the time of construction for cases where groundwater variations are potentially significant for construction. Fluctuation in the groundwater table will occur due to variances in rainfall, elevation, drainage, types of soil encountered and other factors not evident at the time measurements were made. Reference to depth has been made with respect to the existing ground surface encountered at the time of our field investigation. Groundwater levels encountered at each test location at the time of our investigation are shown on the appropriate CPT Sounding and Soil Boring Logs attached in the **Appendix**.



Report of Subsurface Investigation and Geotechnical Engineering Evaluation Proposed BRPD SIM Shooting House Zachary, Louisiana SES Project No: B24-031 April 29, 2024

5.2 Presence of Expansive Soils

Based on the laboratory test results from the soil borings, the subsurface soils encountered within the site are considered to have moderate swelling potential. Generally, the subsurface soils with swelling potential above the ground water level and within the depths subjected to moisture content changes are expected to undergo volume change behavior and were considered in our potential vertical rise (PVR) estimation. The potential vertical rise (PVR) is estimated to be on the order of 1.0 inch using an applied load of 100 psf. The PVR estimates were performed using the TEX 124E method analyzing the upper ten (10) feet of soil.

One (1) inch of PVR is generally accepted as the maximum allowable value for design and construction. However, the Structural Engineer or others should determine if these PVR values are within the acceptable limits. The estimated amount of vertical movement of a foundation or floor slab constructed on swelling clays is referred to as the Potential Vertical Rise (PVR). To reduce the potential for shrinkage and swelling of the site soils, it is important that consideration be given to reducing the potential for moisture changes of the site soils. As a minimum, positive drainage away from the new buildings should be provided. If positive drainage is not provided, water will pond around and/or below the structure and total and differential movements higher than indicated in this report may occur.

6.0 FOUNDATION RECOMMENDATIONS

Our evaluation of foundation conditions has been based on the project information and assumed foundation loadings previously described in this report and subsurface data obtained during the investigation. In evaluating the CPT soundings and auger borings, we have relied on our experience with similar soil conditions and the use of empirical correlations between standard penetration resistances and foundation performance observed in soil conditions similar to those encountered at the subject site.

Soils generally consisted of medium stiff clays. These soils will be suitable for support of lightly loaded structures using a traditional shallow foundation system consisting of square spread and continuous footings near existing grade, provided the allowable bearing capacity and estimated settlement is adequate.

6.1 Site Preparation

Based on our experience with similar site conditions, effective drainage, including ditching and positive grading, should be established across the working areas during the initial stages of site grading, and modified as necessary during construction. Once adequate site drainage is in place, the initial step in site preparation should be the complete removal of topsoil and organic-laden material, stumps and roots, debris, rubble, any existing abandoned foundations or pavement elements, and



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utilities, extending laterally to at least 5 feet outside of the construction area. Excavated materials should be wasted or stockpiled for use in non-structural areas.

Excavation depths required during the initial site preparations to ensure complete removal of all sensitive, fine grained and organic laden material will vary with location and are expected to range from 4 to 6 inches below existing grade. Excavations should extend to a depth of 24 inches below existing grade, 12 inches below the deepest footing elevation, or as needed to remove loose or unsuitable soils that cannot be improved by compaction. Any over-excavated areas should be backfilled using well-compacted structural fill material placed in horizontal lifts as described below.

The exposed subgrade in areas to support pavements, structures, or fill embankments should be proof rolled with a loaded dump truck weighing 15 to 20 tons. Proof-rolling should be performed after a suitable period of dry weather to avoid degrading an otherwise acceptable subgrade and to reduce the amount of undercutting/remedial work required. Any areas that deflect excessively under the equipment and do not stabilize with further compaction should be lowered further by excavation until more stable material is reached.

Excavations made as part of the undercutting or foundation construction should remain open for the least amount of time practical. If soils at the bottom of the excavation are disturbed, saturated, or otherwise become unstable, the excavation should be extended deeper to stable materials. Care should be taken to ensure that any soft or excessively yielding areas are undercut to firmer materials. Any demolished pavements and other debris should be disposed of offsite. Since the lateral and vertical extent may vary, we recommend the excavation and backfilling operation be observed by an experienced soils technician under the direct supervision of the registered project geotechnical engineer of record.

6.2 Structural Fill Recommendations

Fill material should be compacted in 8-inch (maximum) lifts to at least 95 percent of the soil's standard proctor maximum dry density as determined by ASTM D 698. In place density tests should be made at frequent intervals to measure the effectiveness of the compaction operations.

Samples of candidate fill material should be submitted to SES prior to construction to verify that the fill material is suitable for use as structural fill and to establish compaction moisture-density relationships for use as reference for the field density testing.

Imported structural fill material shall be an inert material (non-expansive soil), free of organics (<5%) with a maximum particle size of 2 inches. Commonly available acceptable fill for this region are pumped river sand, lean clays, or clayey sands having the following material properties:



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Soil Type	USCS Classification	Acceptance Criteria
Lean Clay	CL	Liquid Limit less than 40, Plasticity Index between 12 and 22
Pumped River Sand	SP or SP-SM	Less than 10% passing the %200 Sieve
Aggregate Base	GP	LADOTD 610 Crushed Limestone or other similarly graded recycled aggregate

TABLE 1: STRUCTURAL FILL PARAMETERS

6.3 Shallow Foundations

After site preparation operations have been completed as previously discussed, shallow footings bearing at least 18 inches below surrounding grade may be designed for an allowable soil bearing pressure of 1,500 psf. Minimum footing widths of 18 and 24 inches for strip and column footings, respectively, should be observed. Soils exposed in the bottom of footing excavations should consist of medium stiff in-situ soils or acceptable structural fill compacted to a minimum of 95 percent of the standard Proctor maximum dry density to a depth of at least 12 inches below the footing bearing elevation. The bottom of all footing excavations should be observed by an SES engineering technician. Any footing areas found to contain unsuitable materials, poorly compacted soils, or soils that have become softened due to exposure to weather should be undercut and backfilled with new well-compacted structural fill prior to footing construction.

Total settlements for square spread footings up to four (4) feet and continuous footings up to two (2) feet in width and placement of up to two (2) feet of fill material are expected to be less than one (1) inch. Differential settlements can be assumed to be approximately 50% of the total settlement. Settlement was estimated based on the total sustained dead loads of 70% of the above recommended net allowable bearing capacities plus up to two (2) feet of structural fill material, using empirical correlations between CPT data and compressibility. The Structural Engineer shall confirm if these magnitudes are within tolerance limits. If not, SES shall be notified in order to provide some remedial measures and/or change the foundation type.

The bottom of the foundation excavations must be dry, clean and free of loose, soft materials and construction debris prior to placement of steel or concrete. The foundation excavations should be observed by SES's Geotechnical Engineer or their representative prior to steel or concrete placement. Concrete shall be poured as quickly as possible to avoid exposure of the footing materials to moisture changes (wetting or drying). Surface run-off water should be channeled away from the excavation and not be allowed to pond. If for any reason the excavation is required to be open for more than one (1) day, it shall be protected to minimize moisture loss/gain.



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6.3.1 Building Floor Slab

We recommend that a slab-on-grade design can be used to support the building floor slab assuming the site is prepared as described in section 6.1 of this report.

In order to assist with drainage and to increase the modulus of subgrade reaction (k value), a four (4) inch thick free-draining granular mat may be used. The soil surface shall be graded to drain away from the building without low spots that can trap water prior to placing the granular drainage layer. Polyethylene sheeting should be placed to act as a vapor retarder where the floor will be in contact with moisture sensitive equipment or products such as tile, wood, carpet, etc., as directed by the design engineer. The decision to locate the vapor retarder in direct contact with the slab or beneath the layer of granular fill should be made by the design engineer after considering the moisture sensitivity of subsequent floor finishes, anticipated project conditions, and the potential effects of slab curling and cracking. The floor slabs should have an adequate number of joints to reduce cracking resulting from differential movement and shrinkage.

For slabs bearing on properly proof rolled subgrade or at least 12 inches of compacted structural fill, a modulus of subgrade reaction (k value) of 110 pounds per cubic inch (pci) may be used in the grade slab design. If a four (4) inch granular layer is used, a modulus of subgrade reaction (k value) of 130 pounds per cubic inch (pci) may be used.

6.3.2 Grade Supported Slab Precautions

The precautions listed below are for informational purposes for the construction of traditional slab-on-grade pads. These details will not reduce the amount of movement but are intended to reduce potential damage should some settlement of the supporting subgrade take place. Some increase in moisture content is inevitable as a result of development and associated landscaping. However, extreme moisture content increases can be largely controlled by proper and responsible site drainage, building maintenance and irrigation practices.

Cracking of slab-on-grade concrete is normal and should be expected. Cracking can occur not only as a result of heaving or compression of the supporting soil material, but also as a result of concrete curing stresses. The occurrence of concrete shrinkage cracks and problems associated with concrete curing may be reduced and/or controlled by limiting the slump of the concrete, proper concrete placement, finishing, and curing, and by the placement of crack control joints at frequent intervals, particularly where re-entrant slab corners occur. The American Concrete Institute (ACI) recommends a maximum panel size (in feet) equal to approximately three times the thickness of the slab (in inches) in both directions. For example, joints are recommended at a maximum spacing of twelve (12) feet based on having a four-inch slab. SES also recommends that



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the slab be independent of the foundation walls. Using fiber reinforcement in the concrete can also control shrinkage cracking.

Areas supporting slabs should be properly moisture conditioned and compacted. Backfill in all interior and exterior water and sewer line trenches should be carefully compacted to reduce the shear stress in the concrete extending over these areas.

Exterior slabs should be isolated from the building. These slabs should be reinforced to function as independent units. Movement of these slabs should not be transmitted to the building foundation or superstructure.

7.0 PAVEMENT RECOMMENDATIONS

Recommendations for pavement areas are based on laboratory testing performed at the proposed project site and typical vehicular traffic characteristics for a project of this type. These typical minimum pavement sections are based on our experience with similar project assuming mostly passenger vehicle traffic.

Site preparation for parking and drive areas should generally be the same as outlined in Section 6.1 to include complete removal of topsoil, organics, tree root systems, debris, rubble, any existing abandoned foundations or pavement elements, utilities, etc. Tables 2 and 3 outline the minimum recommended pavement section for both rigid and flexible pavement design.

Based on our empirical analysis from field results, a Modulus of Subgrade Reaction (k) of 110 pci should be assigned to the near surface soils. With these assumptions, it is possible to use a typical "standard" pavement section consisting of the following:

Paved Area	Aggregate Base	Concrete ¹ (4000 psi)
Automobile Parking Areas/Standard Duty Drives (passenger vehicles)	8 inches	6 inches
Medium Duty Drives (10,000lb single axle)	8 inches	7 inches
Heavy Duty Drives (18-wheeler, maneuvering areas, dumpster pads)	8 inches	8 inches

TABLE 2: RECOMMENDED MINIMUM RIGID PAVEMENT SECTION

Proper finishing of concrete pavement requires the use of appropriate construction joints to reduce cracking. Construction joints shall be designed in accordance with the current Portland Cement. Association and the American Concrete Institute guidelines (ACI330R-08). Joints should be sealed to reduce the potential for water infiltration into the supporting soils. The design of steel reinforcement should be in accordance with current accepted codes.



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Proposed BRPD SIM Shooting House

Zachary, Louisiana

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April 29, 2024

TABLE 3: RECOMMENDED MINIMUM FLEXIBLE PAVEMENT SECTION

Paved Area	Aggregate Base	Asphalt Surface and Binder
Automobile Parking	8 inches	3 inches
Standard Duty Drives (passenger vehicles)	8 inches	4 inches
Medium Duty Drives (10,000lb single axle)	8 inches	5 inches

7.1 Pavement Base Course

Pavement materials may be placed after the subgrade or structural fill has been properly proof rolled, compacted, and fine-graded. Aggregate base should consist of 610 Limestone or other approved base course Class II base course compacted to at least 95 percent of maximum dry density near the optimum moisture content in accordance with ASTM D698.

Aggregate sections shown for concrete pavement are not required for support, but is recommended to reduce potential for slab curl, shrinkage cracking, and deterioration of subgrade by improving drainage beneath the slab.

7.2 Alternative Base Course Options

As an alternative to the aggregate base shown in Tables 2 and 3, 10-12 inches of lime and/or cement stabilized fill prepared in accordance with Section 305 of the 2016 LSSRB can be substituted for compacted fill and aggregate base. Actual cement and lime addition rates should be based on classification testing performed on actual fill at time of construction in accordance with Section 305.04(a).

7.3 Moisture Control and Drainage for Pavements

One of the most destructive elements that pavement will be subjected to in its design lifetime is the presence of excess moisture. Therefore, pavements should be adequately sloped, and sufficient drainage provided such that excess water is allowed to run off before it can migrate into the pavement system. Sprinkler systems, if utilized in landscaped areas, should be properly installed and aimed such that they do not continually wet the paved surfaces.



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8.0 GENERAL COMMENTS AND LIMITATIONS

While the CPT soundings and auger borings are representative of subsurface conditions at the respective locations and for the respective vertical reaches, local variations characteristic of the subsurface materials of the region are anticipated and may be encountered. The delineation between soil types shown on the logs are approximate and the description represents our interpretation of subsurface conditions at the designated testing location and on the particular date performed.

This report has been prepared in order to aid in the evaluation of this project and to assist the engineers in the structural design. It is intended for use with regard to the specific project discussed herein and any substantial changes in the project, loads, locations, or assumed grades should be brought to our attention so that we may determine how such changes may affect our conclusions and recommendations. We would appreciate the opportunity to review the plans and specifications for construction to ensure that our conclusions and recommendations are interpreted correctly.

Professional judgments on design alternatives and criteria are presented in this report. These are based partly on our evaluations of technical information gathered, partly on our understanding of the characteristics of the project being planned, and partly on our general experience with subsurface conditions in the area. We do not guarantee performance of the project in any respect, only that our engineering work and judgments rendered meet the standard of care of our profession.

As the project geotechnical engineer of record that developed the foundation design recommendations, please be aware that we cannot accept responsibility for the performance of the foundation system if we are not afforded the opportunity to confirm that our recommendations have been followed. Accordingly, we recommend that Southern Earth Sciences, Inc. be retained on this project to perform observation and field-testing services during the construction phase of the foundation system.

This report is exclusively for the use and benefit of the addressee(s) identified on the first page of this report and is not for the use or benefit of, nor may it be relied upon by any other person or entity. The contents of this report may not be quoted in whole or in part or distributed to any person or entity other than the addressee(s) hereof without, in each case, advanced written consent.



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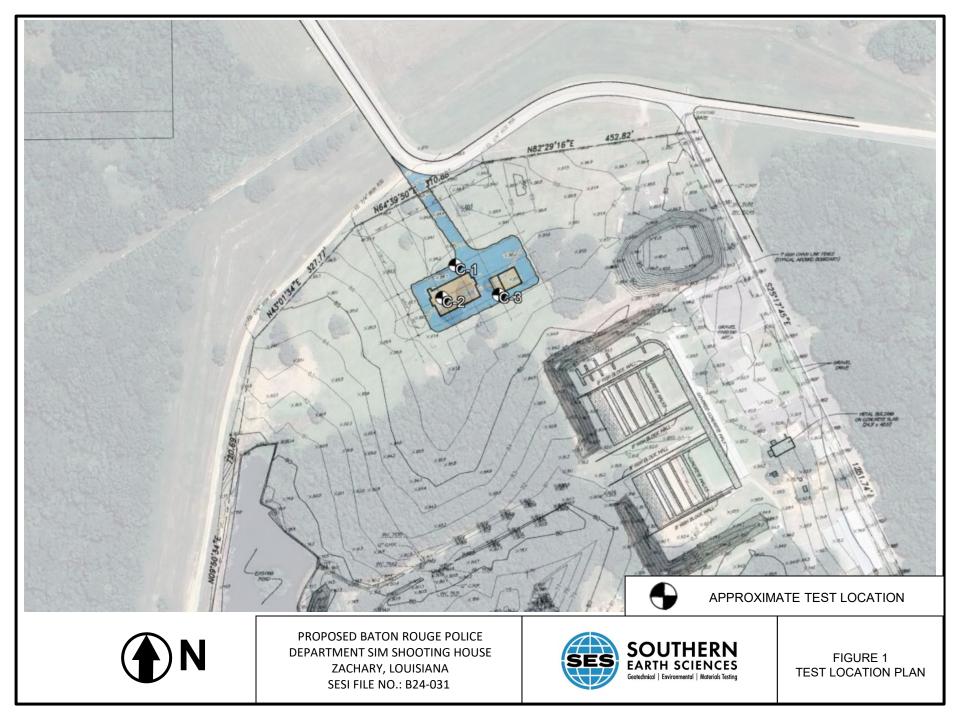
APPENDIX

Test Location Plan

CPT Sounding Logs

Soil Boring Logs

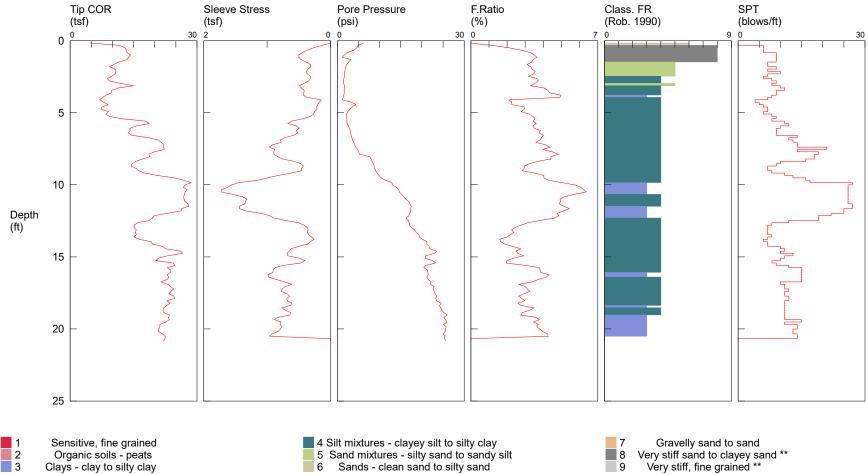






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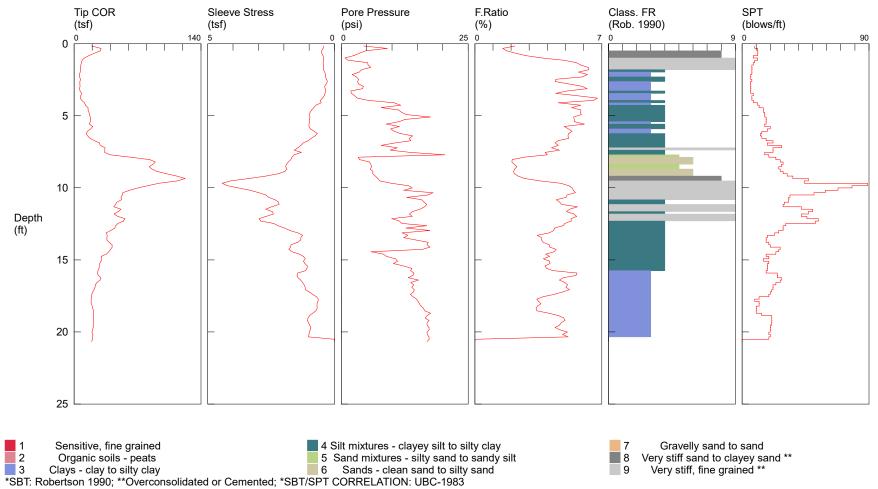
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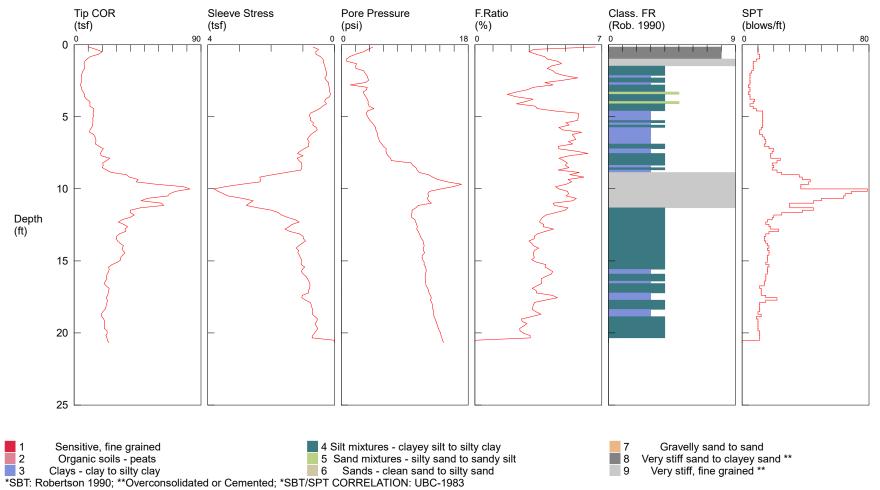


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Baton Rouge, LA 225-356-4355

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> Barry Keller—Project Manager bkeller@soearth.com

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FIELD TESTING AND INSPECTION

In addition to our laboratory testing facilities, SESI maintains a fully outfitted mobile field laboratory available for on-site testing. This allows our OSHA safety certified technicians to perform both call-out services on small projects or full-time quality control testing and inspection on major projects. The on-site testing lab offers a full range of services.

Services

- Dipstick technology for flatness testing of concrete slabs
- Soil testing—compaction, pile load testing, pile and caisson inspection, plate load bearing tests
- Asphaltic concrete testing—core density and thickness, evaluation of aggregates, mix designs, plant and field control
- Portland cement concrete—batch plant and field control, core drilling, molding, curing and testing cylinders
- Slump testing, air content and unit weight
- Pipe and block inspection
- Soundness and abrasion of aggregates
- Bridge inspection
- Pile integrity testing
- Pile dynamic analysis (PDA)
- Vibration monitoring
- Rebar location/depth of cover
- Post tensioning inspection
- Welding and steel framing inspections







LABORATORY TESTING OF MATERIALS

Strategically located laboratories make testing of soils, concrete, asphalt and metals quick and convenient. Branch managers supervise all lab operations in accordance with ASTM Specifications E-329 and E-699. All equipment is calibrated annually to ensure accurate data. SESI technicians are certified by appropriate accrediting agencies on a routine basis.

Services

- Consolidation testing
- Flexible wall permeability testing
- Triaxial testing
- Soil classification testing
- Concrete strength testing
- Steel strength testing



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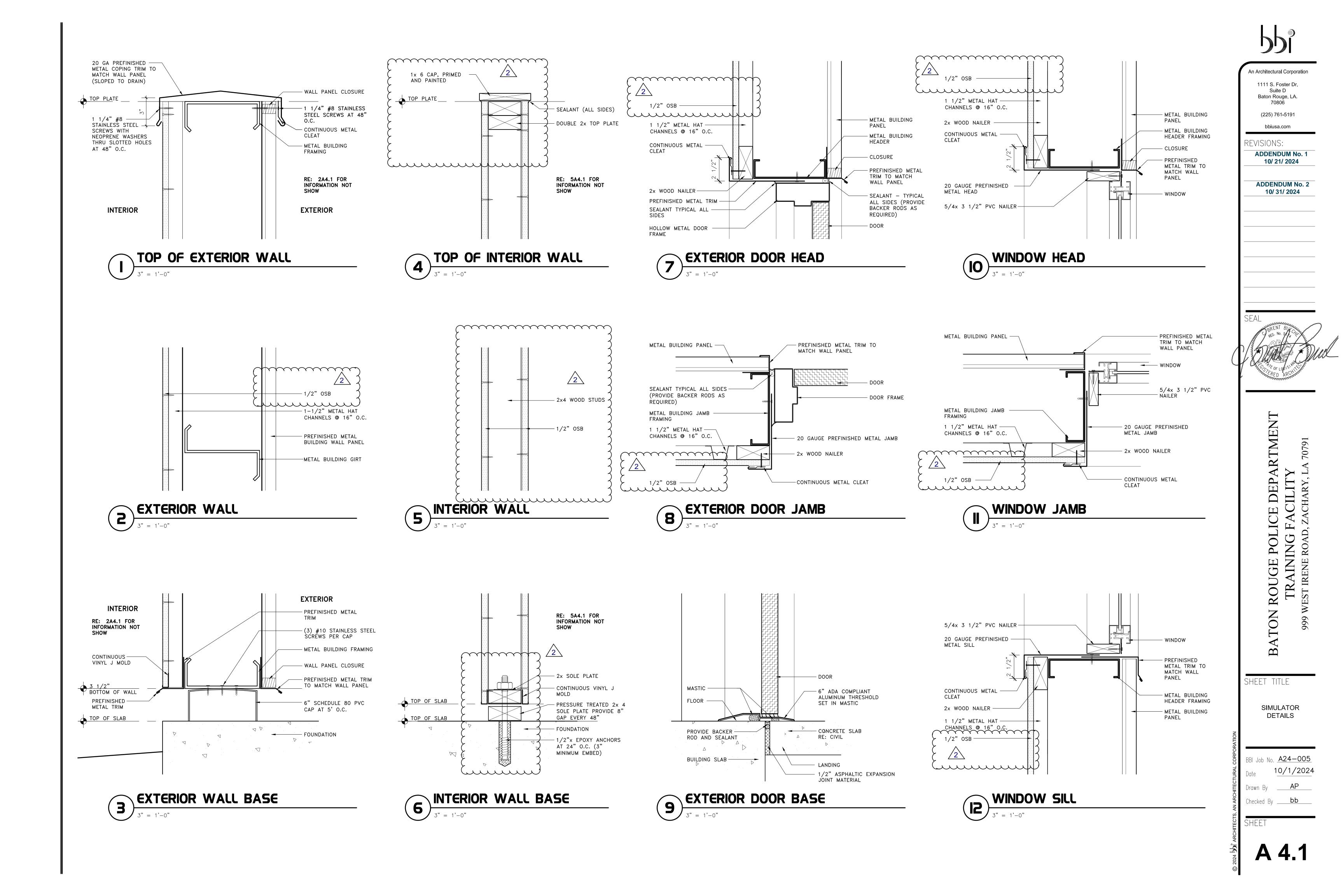
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			42", 36"	OF DEVICE ABOVE FINISH FLOOR OR GRADE.

SHEET NO. SCHEDUL RISER AI GENERAL ELECTRIC CLASSRO SIMULATO E0.0 E0.1 E0.2 E1.0 E2.0 E3.0 E3.1

SHEET INDEX					
SHEET TITLE		15	SSL	JE	
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ULES AND ABBREVIATIONS	0	0			
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AL ELECTRICAL NOTES	0				
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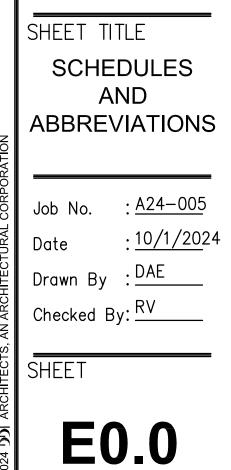


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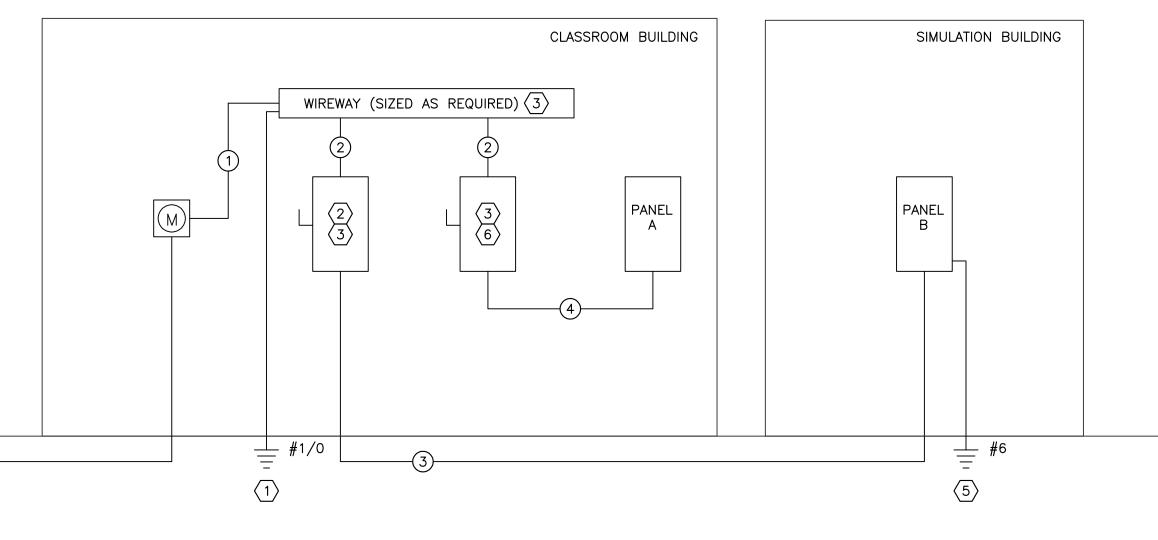






EDS, Inc 24095 ELECTRICAL DESIGN SOLUTIONS (844)EDS-1990 www.ewithd.com

			(FINISH (LUMINA COLORS TO B				ARCHITECT)
LABEL	MANUFACTURER	CATALOG NUMBER	MOUNTING	NOMINAL LUMEN OUTPUT	NOMINAL	CRI	INPUT VOLTS / WATTS	DESCRIPTION
	METALUX	22GR-LD5-32-F1-UNV-L835-CD1 / DF-22W-U						
F1A	LITHONIA LIGHTING	2GTL-F-2-33L-GZ10-LP835 / DGA22	 RECESSED	>=3100	3500K	>=80	120V/ <=25W	2' X 2', LENSED, LED TROFFER WITH DRYWAL
	TRACE-LITE	CBLS-22-CP / LP22-FMK	-					
	METALUX	22GR-LD5-40-F1-UNV-L835-CD1						
F1B	LITHONIA LIGHTING	2GTL-F-2-40L-GZ10-LP835	RECESSED	>=4100	3500K	>=80	120V/ <=33W	2' X 2', LENSED, LED TROFFER WITH 0-10V D
	TRACE-LITE	CBLS-22-CP						
	METALUX	UHBS-1218-MV-L84050	 PENDANT					
F2	LITHONIA LIGHTING	REBL-AL013-MD-UVOLT-SWW9-80CRI-**-M2	@ 17'-8" AFF UNO	>=13000	4000K	>=80	120V/ <=82W	13" DIAMETER, ROUND LED HIGH BAY WITH SELECTABLE CCT ANI
	TRACE-LITE	RSHL-100-CP-**						
	METALUX	UHBS-1218-MV-L84050 / EBP-RM40R REBL-AL013-MD-UVOLT-SWW9-80CRI-**-M2	 PENDANT					13" DIAMETER, ROUND LED HIGH BAY WITH SELECTABLE CCT AND
F2E	LITHONIA LIGHTING	/ RBAY-BLDE40WCP-M4	@ 17'-8" AFF UNO	>=13000	4000K	>=80	120V/ <=82W	40W EMERGENCY DRIVER.
	TRACE-LITE	RSHL-100-CP-**-BB						
	TRACE-LITE	TLED112P-42-VS-4K						
F3		ARC1-LED-P2-40K-MVOLT-**	@ 12'-0" AFF UNO	>=2000	4000K	>=80	120V/ <=27W	LED WALL PACK; WET LOCATION LIS
		CCW-VA1-840-U-T1-**						
F 7 F				>_0000	40001/		1201/ 4 0714	LED WALL BACK WITH INTEODAL EVEDOENCY DATEDY DATE
F3E		ARC1-LED-P2-40K-MVOLT-E4WH	@ 12'-0" AFF UNO	>=2000	4000K	>=80	120V/ <=27W	LED WALL PACK WITH INTEGRAL EMERGENCY BATTERY PAC
	INVUE SURE-LITES	CCW-VA1-840-U-T1-**-EBP 						
X1	LITHONIA LIGHTING	LPXC25 LHQM-LED-R-M6	 PER PLANS	_	_	_	120V / <=5W	SELF-POWERED, POLYCARBONATE, LED COMBINATION EXIT SIGN-EN CADMIUM BATTERY, WHITE BACKGROUND, RED LETTERS, DUAL ADJUS
A1	EXITRONIX	QCRT-R					.207 / \-3₩	LIGHT THROW. PROVIDE FACES AND ARROWS
	SURE-LITES	APEL						
Y1	LITHONIA LIGHTING	EU2L-M12	WALL	_	_	_	120V / <=5W	THERMOPLASTIC, SELF-POWERED, LED EMERGENCY LUMINAIRE WITH N
	EXITRONIX	QMR	@ 7'-6" AFF UNO				,	ADJUSTABLE HEADS.
		# CB TRIP RTG (A) P DESCRIP 1 20 1 REC - CLASSR 3 20 1 REC - CLASSR 5 20 1 REC - CLASSR 5 20 1 REC - CLASSR 7 20 1 REC - CLASSR 9 20 1 REC - STORAG 11 20 1 FAN 1 13 20 1 FAN 2 15 20 1 REC - IT 19 45 2 IWH-2 21 20 1 FLOODSAFE K 25 SPACE SPACE 27 SPACE SPACE 29 SPACE TOTAL CONN TOTAL DIVER TOTAL DIVE (1) (2) (3) (4) (4) (4)	M 720 825 M 720 825 M 360 8328 DR 900 8328 DR 696 3120 EE 900 3120 696 3120 1 4150 2050 1 1T 2050 1 ECTED VA 35147 1 NECTED A 336 SIFIED VA 48487 RSIFIED A 233	PHASE B DESCRIPTION P VA LTG - INTERIOR 1 900 281 LTG - EXTERIOR 1 900 281 LTG - EXTERIOR 1 720 8328 AHU-1 2 696 8328 AHU-2 2 696 8328 HP-1 2 500 3120 HP-2 2 4150 3120 HP-1 2 600 2050 IWH-1 2 600 2050 IWH-1 2 34843 PANEL GENERAL NOTES I	CB TRIP # 20 2 20 4 90 6 8 90 90 10 12 30 30 18 20 22 24 25 25 22 24 25 30 30	3 20 1 RI 5 20 1 F/ 7 20 1 F/ 9 20 1 F/ 11 20 1 EI 13 20 1 EI 15 SI SI 17 SI SI 19 SI SI 21 SI SI 23 SI SI CIRCUIT NOTES (1) PROVIDE GFC (2) (3) (4)	DESCRIPTION EC - CAT WALK (1) AN 1 AN 1 AN 1 AN 2 AN 3 1000 WC F-1 F-1 4() PACE PACE PACE PACE PACE PACE PACE PACE PACE PACE PACE PACE PACE PACE CONNECTED VA TOTAL CONNECTED A TOTAL DIVERSIFIED VA TOTAL DIVERSIFIED A CI CIRCUIT BREAKER.	720 758 LTG - INTERIOR SOUTH 1 20 4 6 135 LTG - EXTERIOR NORTH 1 20 6 696 162 LTG - EXTERIOR SOUTH 1 20 8
		EXISTING UTILITY POLE WITH XFMR	NEW UTILITY POLE.		WIREWA	Y (SIZED AS R	CLASSROOM	A BUILDING PANEL A
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## **GENERAL NOTES:**

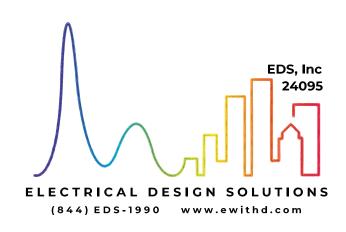
1. PROVIDE PULL BOXES AS PER MANUFACTURER'S RECOMMENDATIONS TO MEET MANUFACTURER REQUIREMENTS FOR MAXIMUM PULL LENGTHS.

### SPECIFIC NOTES:

- $\langle 1 \rangle$  MAIN SERVICE GROUND. PROVIDE COPPER GROUND, SIZED AS SHOWN, PER NEC 250.52.
- (2) 100A/100AF/2P,3W/240V/N-3R SERVICE RATED DISCONNECT SWITCH. FUSES SHALL BE BUSSMANN LPN-RK-100SP.
- $\overline{3}$  PROVIDE NEUTRAL TO GROUND BONDING PER NEC 250.
- COORDINATE WITH UTILITY TO PROVIDE RACEWAY AND/OR WIRING PER THEIR REQUIREMENTS.
- 5 PROVIDE GROUNDING, SIZED AS SHOWN, FOR A STRUCTURE BEING SUPPLIED BE A FEEDER FROM A SEPARATE STRUCTURE PER NEC 250.32.
- 6 400A/250AF/2P,3W/240V/N-3R SERVICE RATED DISCONNECT SWITCH. FUSES SHALL BE BUSSMANN LPN-RK-250SP.

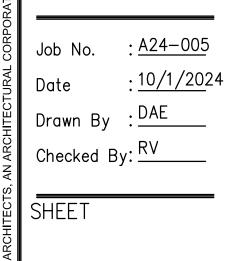
### <u>FEEDERS</u>

- 1) 2 SETS OF [3 #3/0 & 1 #1/0(G) IN 2"R.]
- ② 3 #1 & 1 #1/0(G) IN 1−1/2"R.
- ③ 3 #1 & 1 #8(G) IN 1−1/2"R.
- ④ 3 #250MCM & 1 #4(G) IN 2−1/2"R.



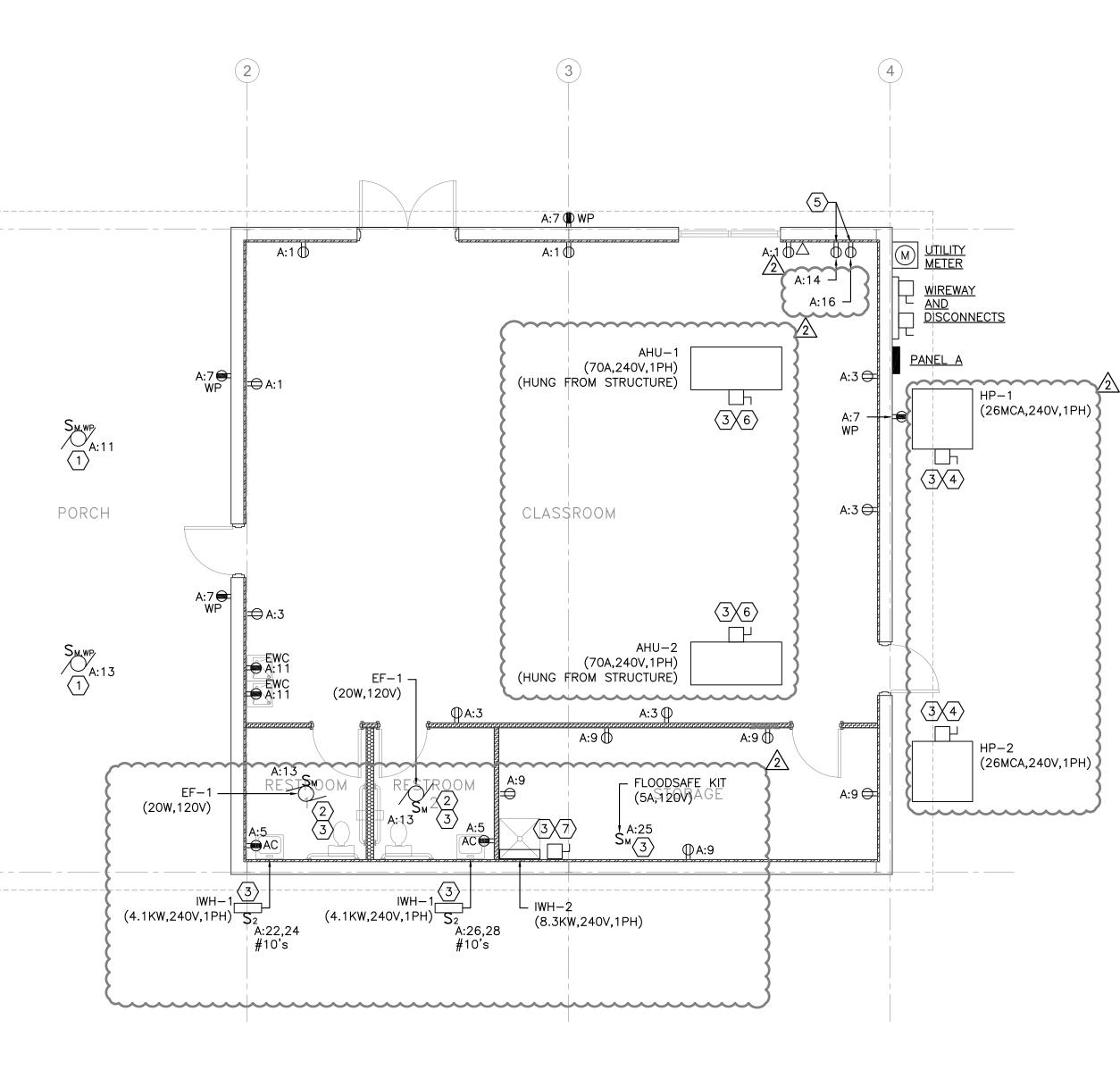
SHEET TITLE

### **RISER AND** SCHEDULES



E0.1

( A )-(B)-





### **GENERAL NOTES:**

- 1. BRANCH CIRCUIT HOMERUNS SHALL BE SIZED FOR VOLTAGE DROP BASED UPON THE FOLLOWING FOR 120V, 1ø, 20A LOADS:
  - #12 0'-70' #10 - 71'-110' #8 - 111'-175'#6 - 176'-280'

PROVIDE APPROPRIATE HARDWARE TERMINATION FOR ASSOCIATED WIRE SIZE.

- 2. BRANCH CIRCUITS SHALL NOT EXCEED 3% VOLTAGE DROP. UPSIZE WIRE AS REQUIRED.
- SEE ARCHITECTURAL DRAWINGS FOR EXACT DEVICE LOCATIONS AND MOUNTING HEIGHTS PRIOR TO ROUGH-IN.

### SPECIFIC NOTES:

- 1 FOR FAN (1/4HP, 120V). VERIFY EXACT LOCATION OF CONTROLLER WITH OWNER AND PROVIDE WIRING/CABLING AND RACEWAY FROM CONTROLLER TO FAN PER MANUFACTURER'S REQUIREMENTS.
- 2 PROVIDE ON/OFF CONTROL OF EXHAUST FAN VIA SWITCH WITH OCCUPANCY SENSORS THAT CONTROLS LIGHTING WITHIN THIS ROOM.
- 3 CONTRACTOR SHALL VERIFY EXACT ELECTRICAL REQUIREMENTS AND COORDINATE THE EXACT LOCATION WITH MECHANICAL PRIOR TO ROUGH-IN AND INSTALLATION. CONTRACTOR SHALL PROVIDE ANY REQUIRED RACEWAY, FEEDER, DISCONNECT DEVICES, AND ANY OTHER ACCESSORIES OR MATERIALS NOT MENTIONED THAT ARE NEEDED FOR THE SAFE AND PROPER FUNCTIONING OF THE EQUIPMENT.
- 30A/NF/2P,2W/240V/N-3R DISCONNECT SWITCH FOR HP. PROVIDE 2 #10 & 1#10(G) IN 3/4"R BRANCH CIRCUIT.
   INSTALL RECEPTACLE ON TELEPHONE BACKBOARD. VERIFY EXACT LOCATION WITH OWNER PRIOR TO ROUGH-IN.
   100A/NF/2P,2W/240V/N-3R DISCONNECT SWITCH
- FOR AHU. PROVIDE 2 #4 & 1#8(G) IN 1"R BRANCH CIRCUIT.
  60A/NF/2P,2W/240V/N-3R DISCONNECT SWITCH FOR IWH-2. PROVIDE 2 #6 & 1#10(G) IN 3/4"R BRANCH CIRCUIT.

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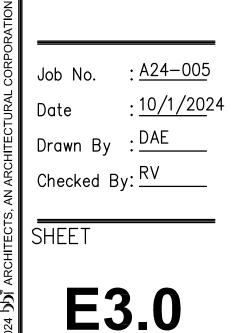
REVISIONS: 11/01/24 addendum 2



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